

**BLACK & VEATCH**

**South Florida Water Management District  
EAA Reservoir A-1 Basis of Design Report**

**January 2006**

**APPENDIX 8-4**

**RCC STABILITY ANALYSIS  
EMBANKMENT ALTERNATIVE NO. 5**

South Florida Water Management District  
**EAA Reservoir A-1 Basis of Design Report**

January 2006

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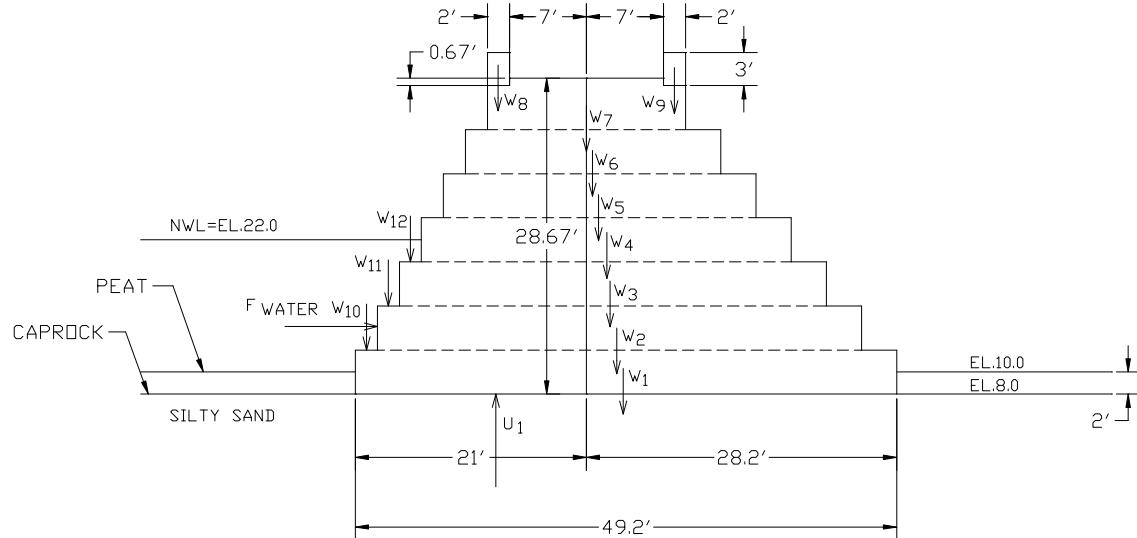
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# RCC Stability Analysis Embankment Alternative 5

## 1. STABILITY ANALYSIS TO USACE EM 1110-2-2200

### 1.1 Usual Loading Condition – Normal water level, minimum tail water

#### 1.1.1 Overturning Stability



EMBANKMENT ALTERNATIVE NO. 5  
ROLLER COMPACTED CONCRETE-RCC

$$\sum M_{TOE} = W_{12}d_{12} + W_{11}d_{11} + W_{10}d_{10} + W_9d_9 + W_8d_8 + W_7d_7 + W_6d_6 + W_5d_5 + W_4d_4 + W_3d_3 + W_2d_2 + W_1d_1 - 0.5\gamma d^2 d_y \\ - 0.5\gamma d_{total}x_{base}x_{toe}$$

$$W = V\rho$$

$$Ex: \quad W_1 = (4')(49.2')(1')(138 \frac{lb}{ft^3}) = 27,158 \text{ lb}$$

$$\sum M_{TOE} = (249.6)(44.2) + (748.8)(46.2) + (1248)(48.2) + (828)(20.2) + (828)(36.2) + (11230)(28.2) + (12806)(27.6) \\ + (15677)(27) + (18547)(26.4) + (21418)(25.8) + (24288)(25.2) + (27158)(24.6) - 0.5(62.4)(12)^2(6) - 0.5(62.4)(14)(49.2)(32.8) \\ = 2,836,413 \text{ lb-ft}$$

$$\sum V = W_{12} + W_{11} + W_{10} + W_9 + W_8 + W_7 + W_6 + W_5 + W_4 + W_3 + W_2 + W_1 - U_1 \\ = 249.6 + 748.8 + 1248 + 828 + 828 + 11230 + 12806 + 15677 + 18547 + 21418 + 24288 + 27158 - 21491 \\ = 113,535 \text{ lb}$$

$$\text{Resultant Location} = \frac{\sum M}{\sum V} \\ = \frac{2836413}{113535} \\ = 24.98 \text{ ft}$$

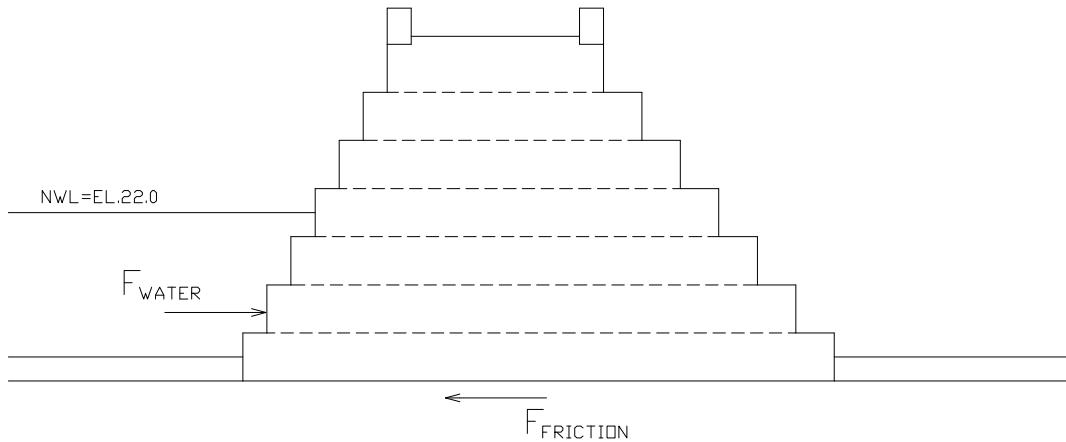
Requirement: Usual Load Condition requires resultant location in Middle 1/3

$$\text{Middle } 1/3 = 16.4 \text{ ft} < x < 32.8 \text{ ft}$$

Therefore, resultant location meets requirement

## RCC Stability Analysis Embankment Alternative 5

### 1.1.2 Sliding



$$\begin{aligned}
 \text{F.S.} &= \frac{T_f}{T} \\
 &= \frac{\mu W_{\text{TOTAL}}}{F_{\text{WATER}}} \\
 &= \frac{\mu W_t}{\gamma_w d^2} \\
 &= \frac{(0.7)(113535)}{0.5(62.4)(12)^2} \\
 &= 17.69
 \end{aligned}$$

Requirement: Usual Load Condition requires factor of safety > 2.0

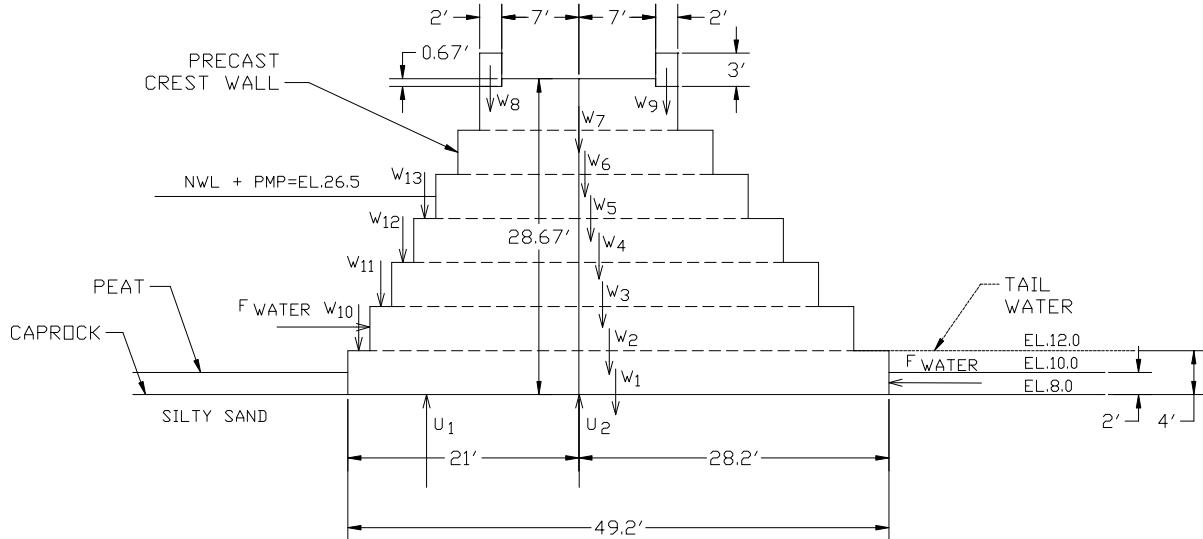
$$17.69 > 2.0$$

Therefore, sliding requirement is met.

## RCC Stability Analysis Embankment Alternative 5

### 1.2 Extreme Loading Condition – Water level at maximum flood (PMP), tail water pressure

#### 1.2.1 Overturning



EMBANKMENT ALTERNATIVE NO. 5  
ROLLER COMPAKTED CONCRETE-RCC

$$\sum M_{TOE} = W_{13}d_{13} + W_{12}d_{12} + W_{11}d_{11} + W_{10}d_{10} + W_9d_9 + W_8d_8 + W_7d_7 + W_6d_6 + W_5d_5 + W_4d_4 + W_3d_3 + W_2d_2 + W_1d_1 + 0.5\gamma d^2 d_{y1} - 0.5\gamma d^2 d_{y2} - (0.5\gamma d_{base}(d_{water_1} - d_{water_2})) \left( \frac{2d_{base}}{3} \right) - 0.5\gamma d_{water_{21}} d_{base} \left( \frac{d_{base}}{2} \right)$$

$$W = Vp$$

$$Ex: W_7 = (4')(49.2')(1')(138 \text{ lb/ft}^3) = 27158 \text{ lb}$$

$$\begin{aligned} \sum M_{TOE} &= (312)(42.2) + (811.2)(44.2) + (1310.4)(46.2) + (1809.6)(48.2) + (828)(20.2) + (828)(36.2) \\ &\quad + (11230)(28.2) + (12806)(27.6) + (15677)(27) + (18547)(26.4) + (21418)(25.8) + (24288)(25.2) \\ &\quad + (27158)(24.6) + 0.5(62.4)(2)^2 \left( \frac{4}{3} \right) - 0.5(62.4)(16.5)^2 (5.5) - 0.5(62.4)(49.2)(14 - 4)(32.8) - 0.5(62.4)(49.2)(4)(49.2) \\ &= 2,958,877 \text{ lb-ft} \end{aligned}$$

$$\begin{aligned} \sum V &= W_{14} + W_{13} + W_{12} + W_{11} + W_{10} + W_9 + W_8 + W_7 + W_6 + W_5 + W_4 + W_3 + W_2 + W_1 - U_1 - U_2 \\ &= 399.4 + 312 + 811.2 + 1310.4 + 1809.6 + 828 + 828 + 11230 + 12806 + 15677 + 18547 + 21418 + 24288 + 27158 - 15350 - 6140 \\ &= 115,933 \text{ lb} \end{aligned}$$

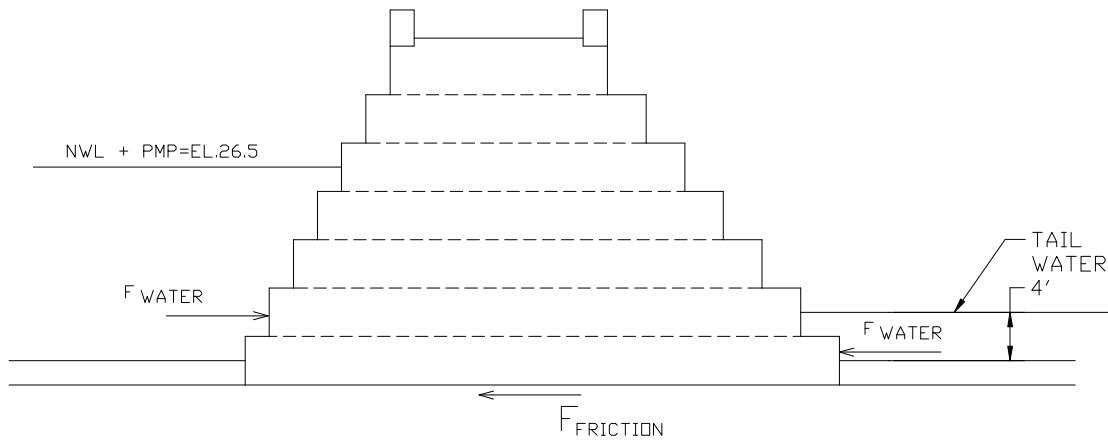
$$\begin{aligned} \text{Resultant Location} &= \frac{\sum M}{\sum V} \\ &= \frac{2958877}{115933} \\ &= 25.52 \text{ ft} \end{aligned}$$

Requirement for Extreme Load Condition: Resultant must be within base  
Therefore, resultant location meets requirement

## RCC Stability Analysis Embankment Alternative 5

### 1.2.2

### *Sliding*



$$\begin{aligned}
 \text{F.S.} &= \frac{T_f}{T} \\
 &= \frac{\mu W_{\text{TOTAL}}}{F_{\text{WATER}}} \\
 &= \frac{\mu W_t}{\gamma_w d_1^2 - \gamma_w d_2^2} \\
 &= \frac{(0.7)(115933)}{0.5(62.4)(16.5)^2 - 0.5(62.4)(2)^2} \\
 &= 9.70
 \end{aligned}$$

Design requirement for extreme load condition: factor of safety  $> 1.3$

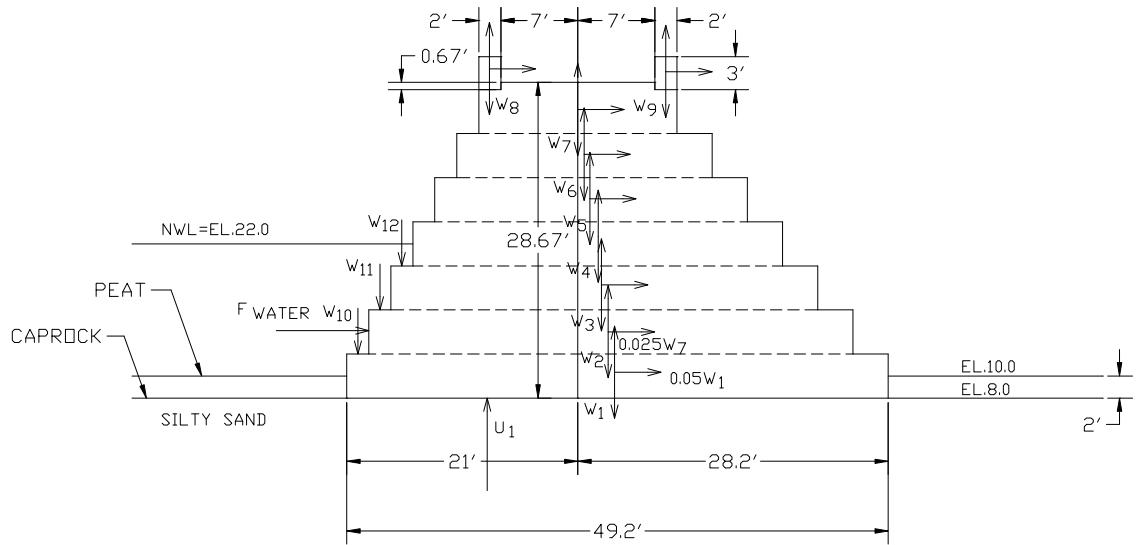
$$9.70 > 1.3$$

Therefore, requirement is met.

# RCC Stability Analysis Embankment Alternative 5

## 1.3 Seismic Conditions – Normal water level, maximum credible earthquake, no tail water

### 1.3.1 Overturning



EMBANKMENT ALTERNATIVE NO. 5  
ROLLER COMPACTED CONCRETE-RCC

## RCC Stability Analysis Embankment Alternative 5

$$\begin{aligned}\sum M_{TOE} = & W_{12}d_{x12} + W_{11}d_{x11} + W_{10}d_{x10} + W_9d_{x9} + W_8d_{x8} + W_7d_{x7} + W_6d_{x6} + W_5d_{x5} + W_4d_{x4} + W_3d_{x3} + W_2d_{x2} + W_1d_{x1} \\ & + 0.05W_9d_{y9} + 0.05W_8d_{y8} + 0.05W_7d_{y7} + 0.05W_6d_{y6} + 0.05W_5d_{y5} + 0.05W_4d_{y4} + 0.05W_3d_{y3} + 0.05W_2d_{y2} \\ & + 0.05W_1d_{y1} + 0.025W_9d_{x9} + 0.025W_8d_{x8} + 0.025W_7d_{x7} + 0.025W_6d_{x6} + 0.025W_5d_{x5} + 0.025W_4d_{x4} + 0.025W_3d_{x3} \\ & + 0.025W_2d_{x2} + 0.025W_1d_{x1} - 0.5\gamma d^2 d_y - 0.5\gamma d_{total} x_{base} d_{toe}\end{aligned}$$

$$W = Vp$$

$$Ex: \quad W_7 = (4')(49.2')(1')\left(\frac{138 \text{ lb}}{\text{ft}^3}\right) = 27,158 \text{ lb}$$

$$\begin{aligned}\sum M_{TOE} = & (249.6)(44.2) + (748.8)(46.2) + (1248)(48.2) + (828)(20.2) + (828)(36.2) + (11230)(28.2) + (12806)(27.6) \\ & + (15677)(27) + (18547)(26.4) + (21418)(25.8) + (24288)(25.2) + (27158)(24.6) - 0.05(27158)(2) - 0.05(24288)(6) \\ & - 0.05(21418)(10) - 0.05(18547)(14) - 0.05(15677)(18) - 0.05(12806)(22) - 0.05(11230)(26) - 2[0.05(828)(29.5)] \\ & - 0.025(828)(20.2) - 0.025(828)(36.2) - 0.025(11230)(28.2) - 0.025(12806)(27.6) - 0.025(15677)(27) \\ & - 0.025(18547)(26.4) - 0.025(21418)(25.8) - 0.025(24288)(25.2) - 0.025(27158)(24.6) - 0.5(62.4)(12)^2(6) \\ & - 0.5(62.4)(14)(49.2)(32.8) \\ = & 2,670,919 \text{ lb-ft}\end{aligned}$$

$$\begin{aligned}\sum V = & W_{12} + W_{11} + W_{10} + W_9 + W_8 + W_7 + W_6 + W_5 + W_4 + W_3 + W_2 + W_1 - 0.025W_9 - 0.025W_8 - 0.025W_7 - U_1 \\ & - 0.025W_6 - 0.025W_5 - 0.025W_4 - 0.025W_3 - 0.025W_2 - 0.025W_1 \\ = & 249.6 + 748.8 + 1248 + 828 + 828 + 11230 + 12806 + 15677 + 18547 + 21418 + 24288 + 27158 - 20.7 - 20.7 - 280.8 \\ & - 320.2 - 391.9 - 463.7 - 535.5 - 607.2 - 679.0 - 21490.6 \\ = & 110,216 \text{ lb}\end{aligned}$$

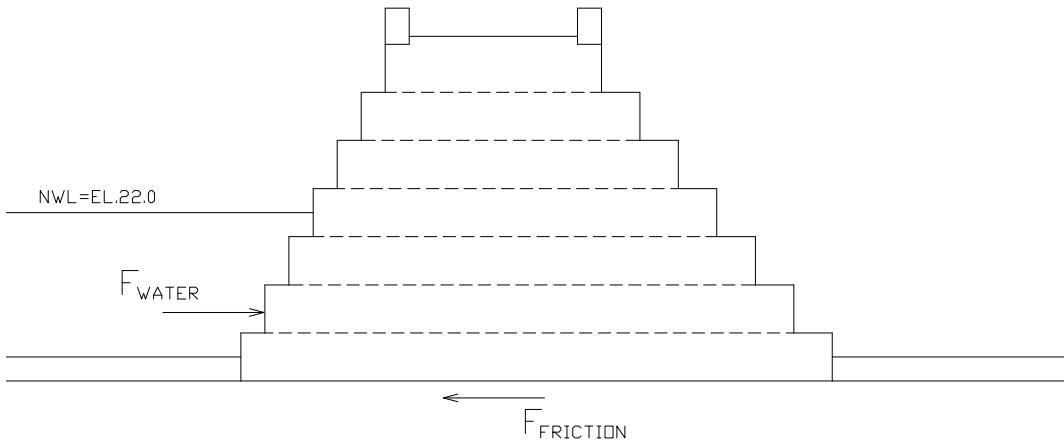
$$\begin{aligned}\text{Resultant Location} &= \frac{\sum M}{\sum V} \\ &= \frac{2670919}{110216} \\ &= 24.23 \text{ ft}\end{aligned}$$

Design requirement for Extreme Load Condition: Resultant location within base

Therefore, design requirement is met.

## RCC Stability Analysis Embankment Alternative 5

### 1.3.2 Sliding



$$\begin{aligned}
 \text{F.S.} &= \frac{T_f}{T} \\
 &= \frac{\mu W_{\text{TOTAL}}}{F_{\text{WATER}}} \\
 &= \frac{\mu W_t}{\gamma_w d^2} \\
 &= \frac{(0.7)(110216)}{0.5(62.4)(12)^2} \\
 &= 17.17
 \end{aligned}$$

Design requirement for extreme load condition: Factor of safety  $> 1.3$

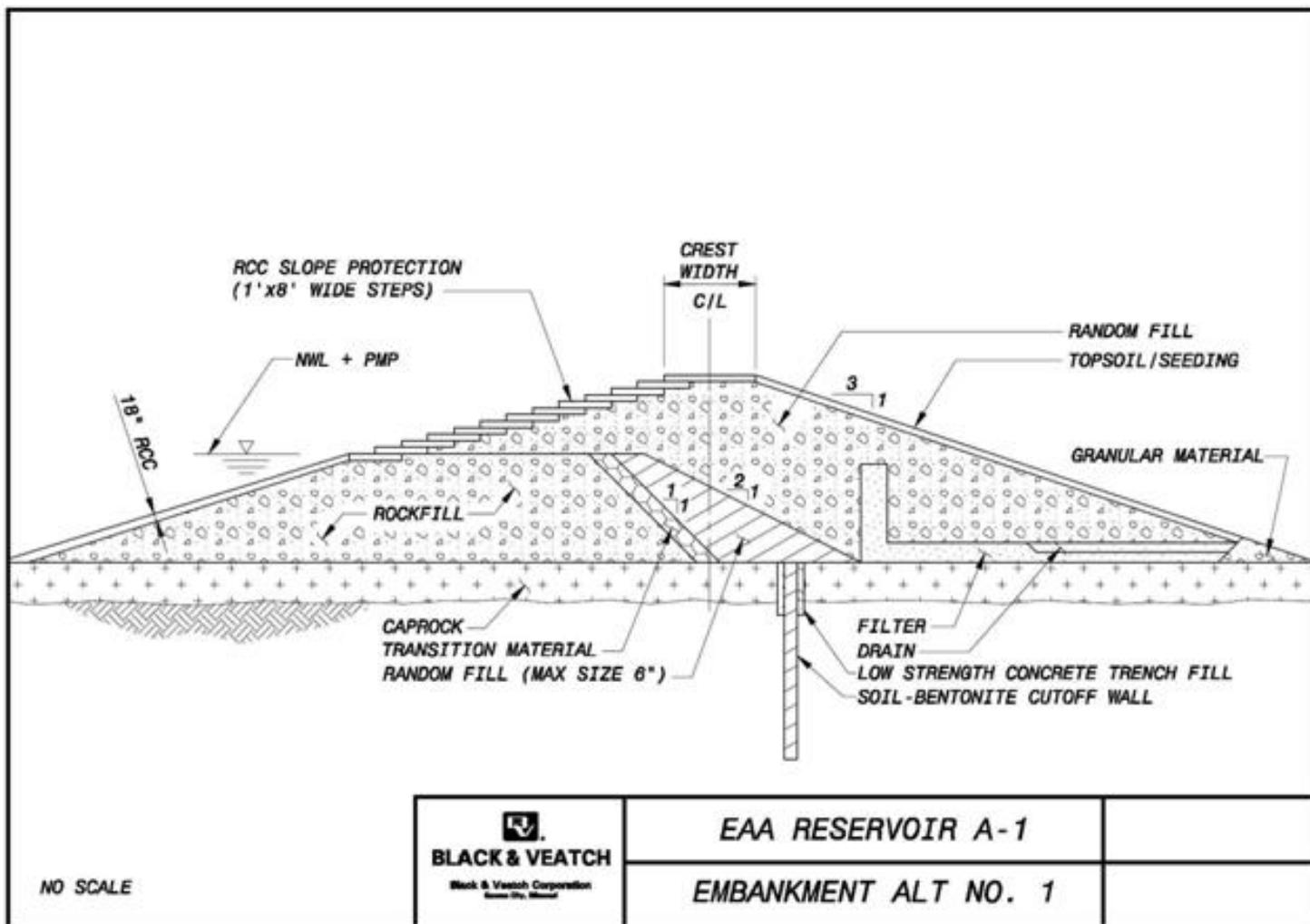
$$17.17 > 1.3$$

Therefore, design requirement is met.

## RCC Stability Analysis Embankment Alternative 5

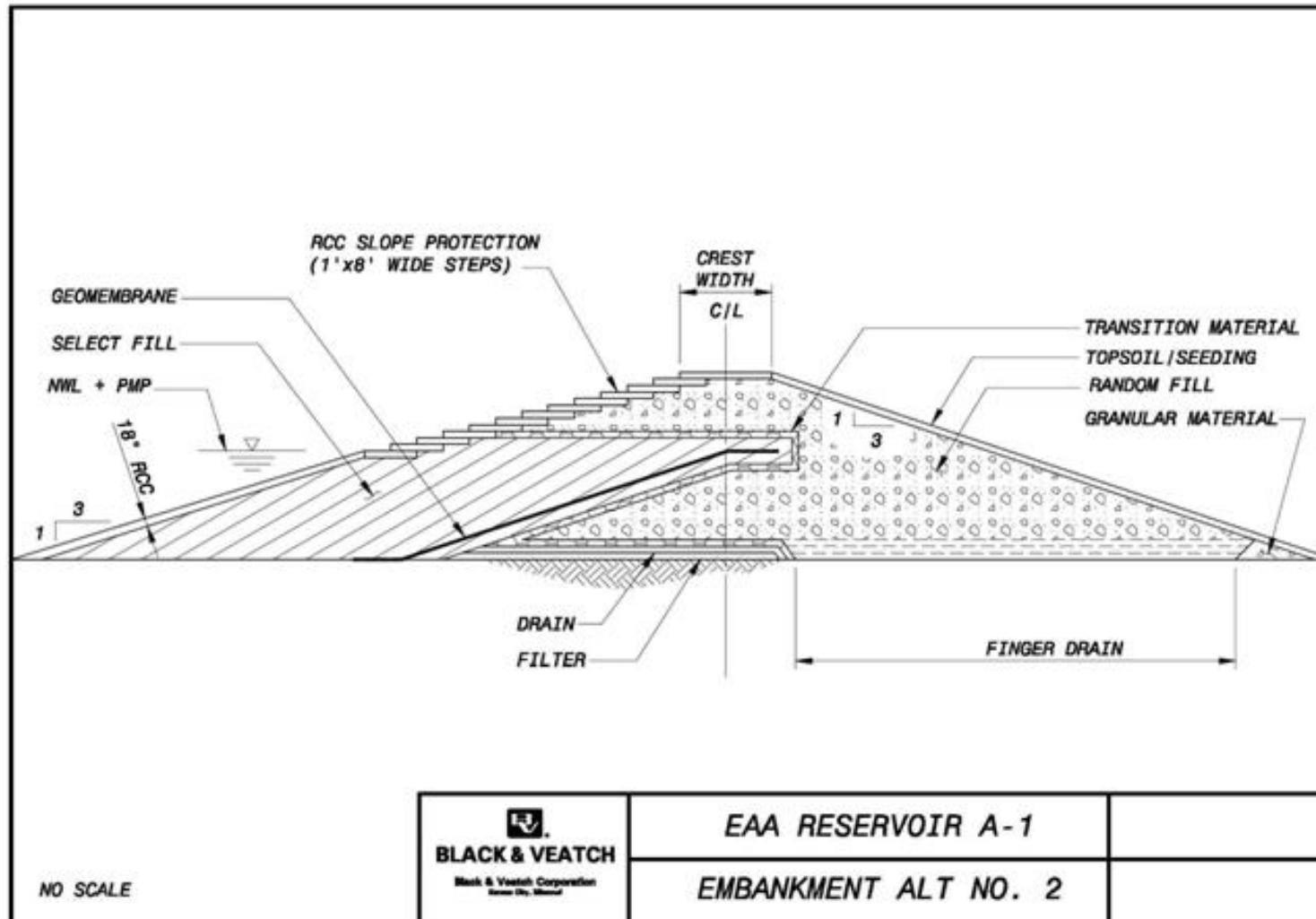
### FIGURES

Figure 1 Embankment Alternative 1



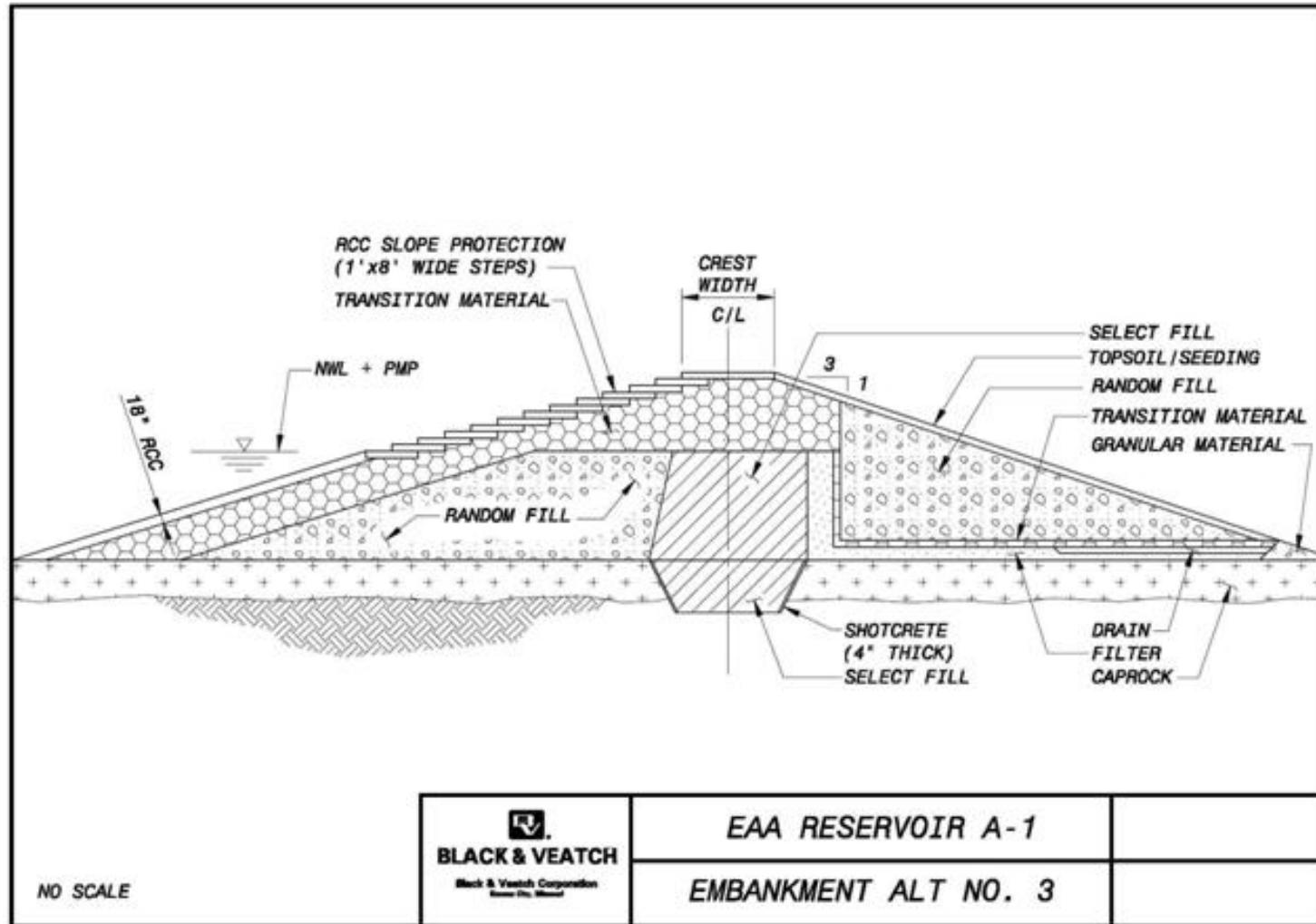
## RCC Stability Analysis Embankment Alternative 5

Figure 2 Embankment Alternative 2



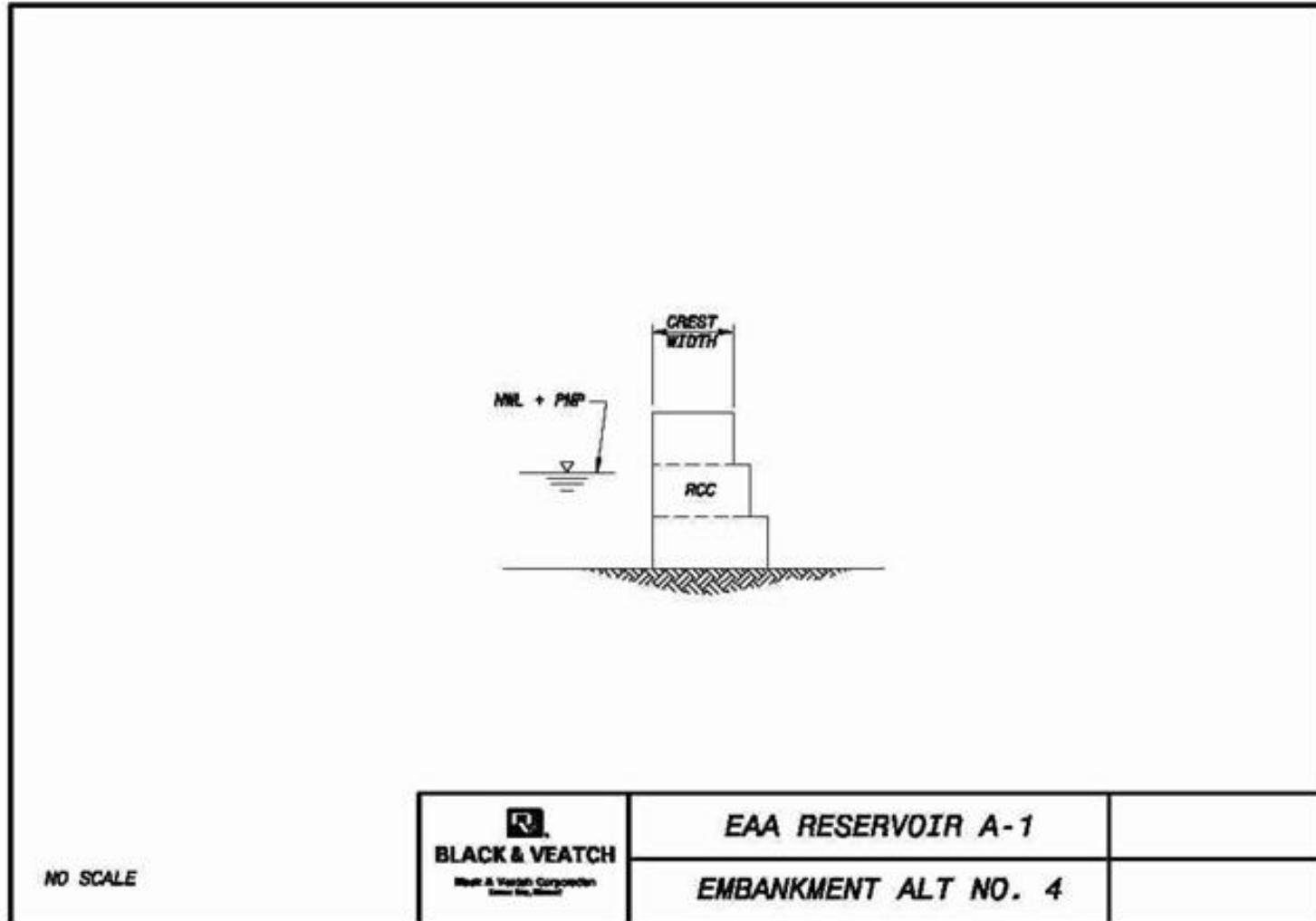
## RCC Stability Analysis Embankment Alternative 5

Figure 3 Embankment Alternative 3



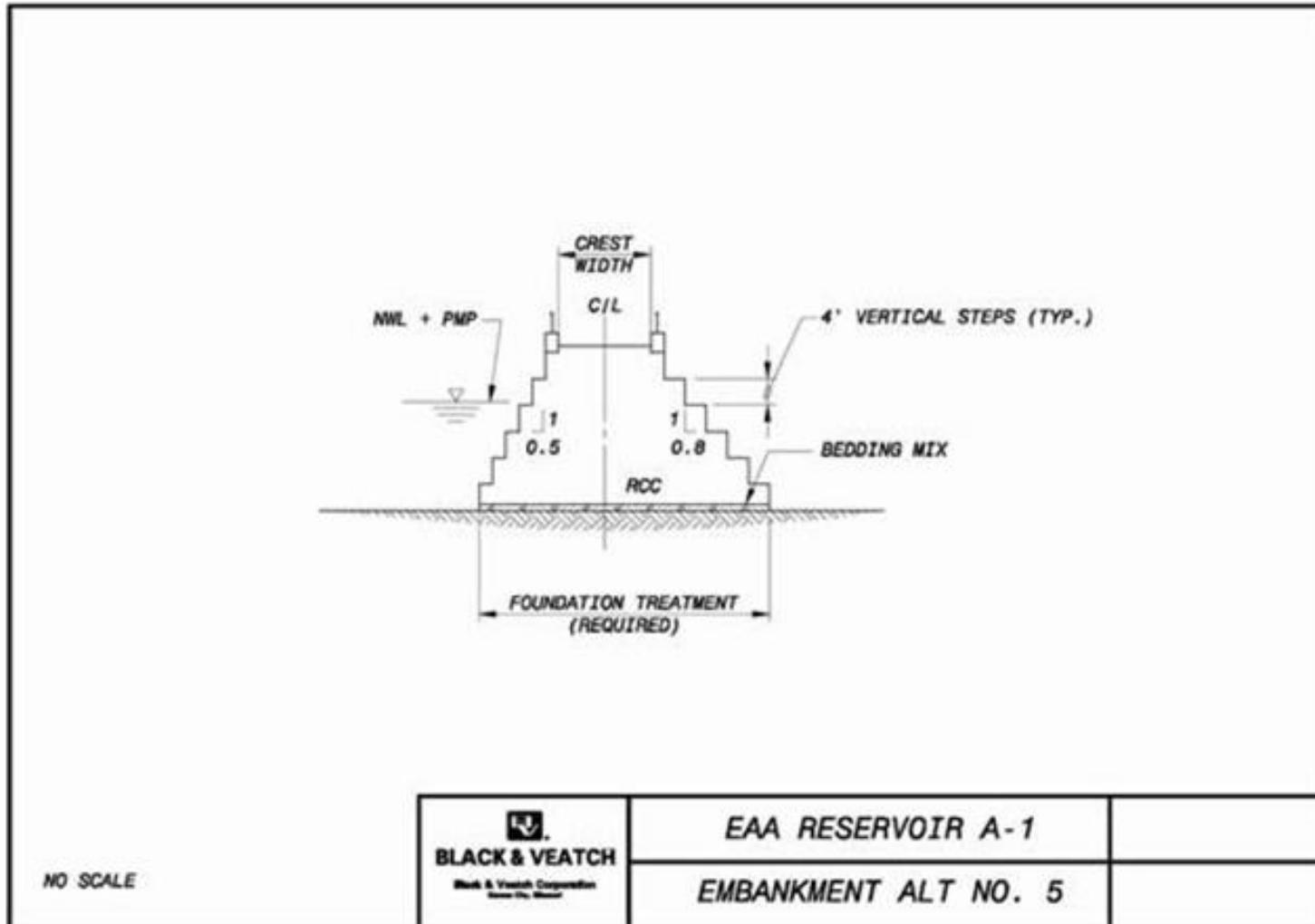
## RCC Stability Analysis Embankment Alternative 5

Figure 4      Embankment Alternative 4



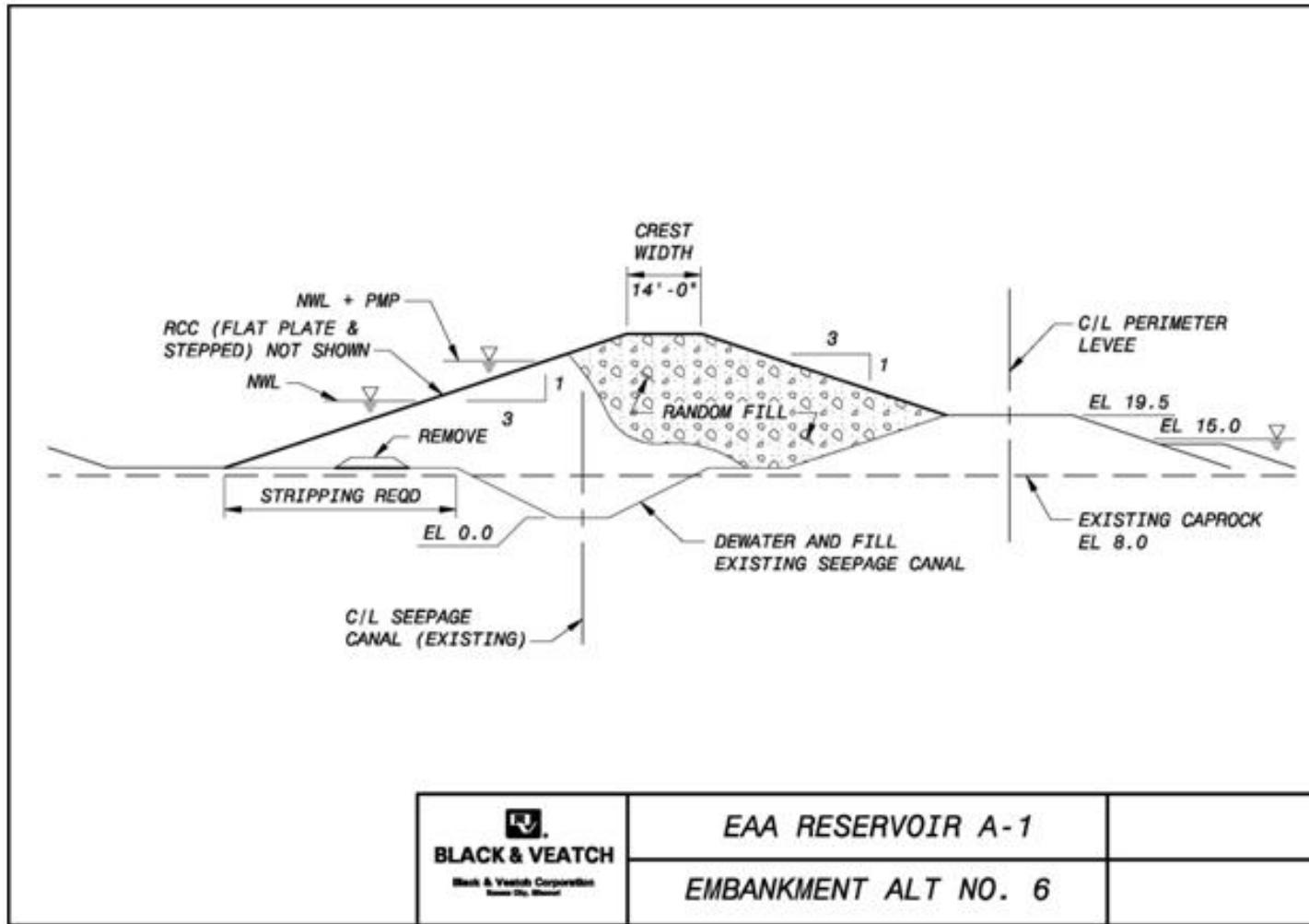
## RCC Stability Analysis Embankment Alternative 5

Figure 5 Embankment Alternative 5



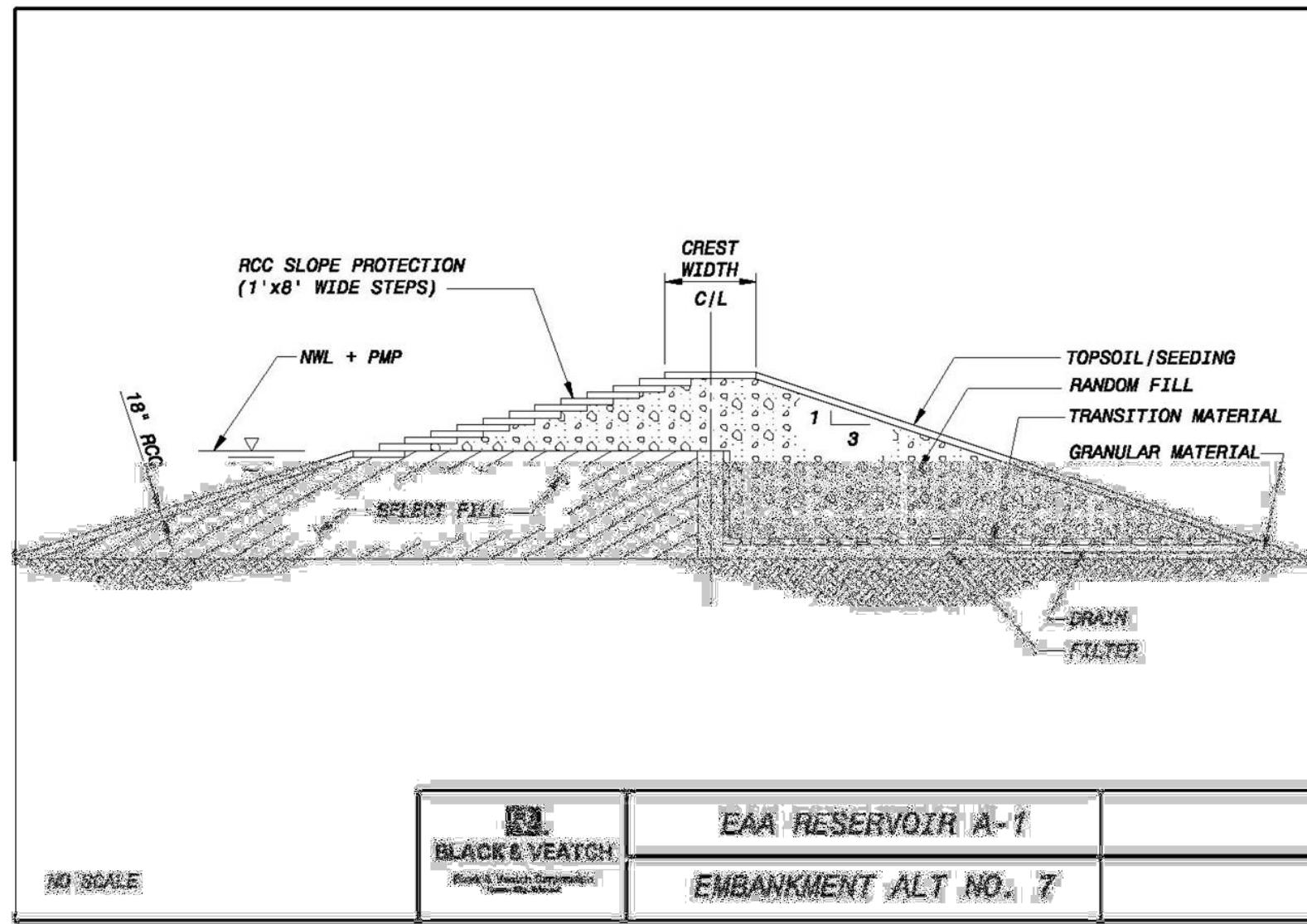
## RCC Stability Analysis Embankment Alternative 5

Figure 6 Embankment Alternative 6



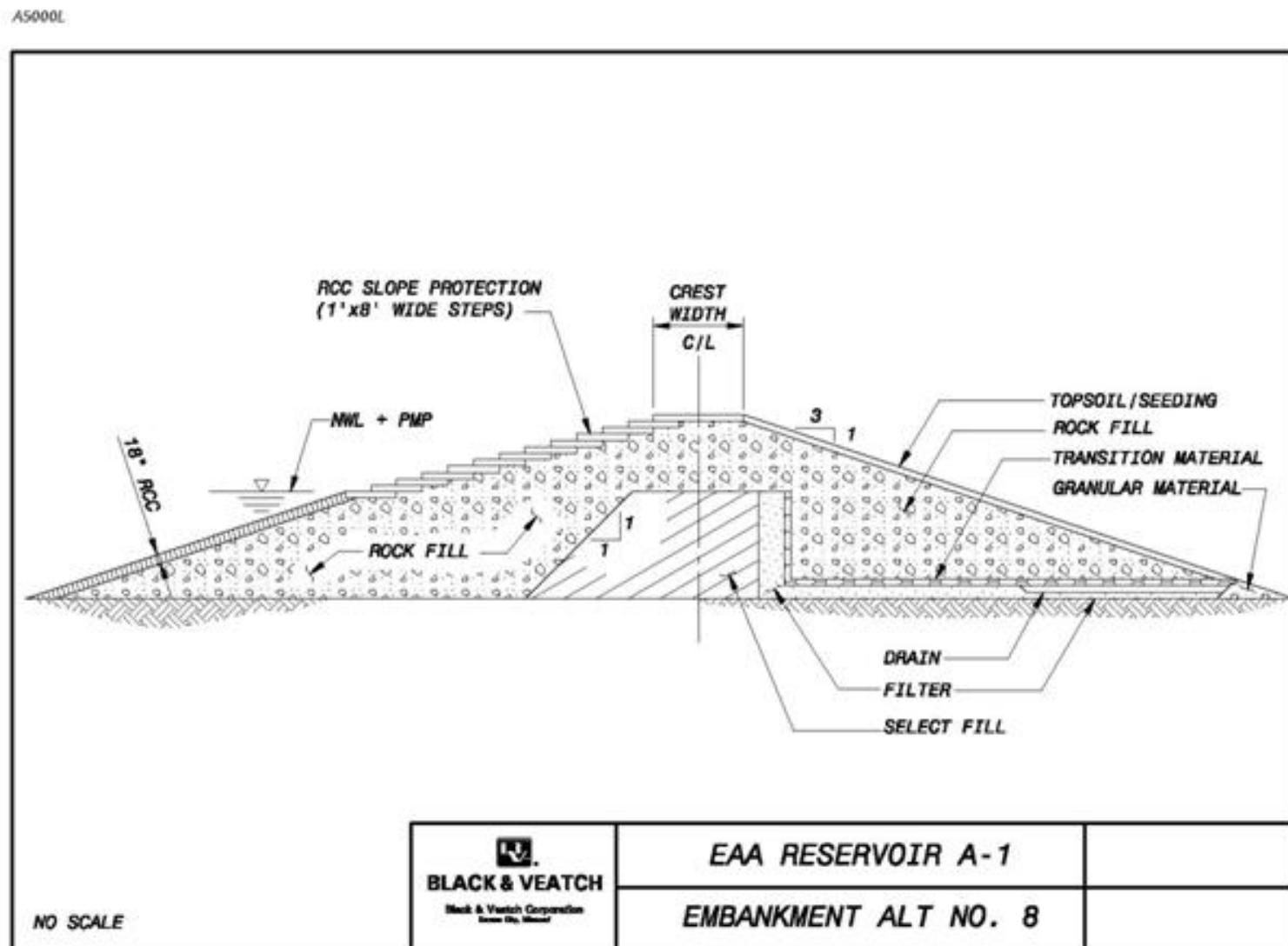
## RCC Stability Analysis Embankment Alternative 5

Figure 7 Embankment Alternative 7



## RCC Stability Analysis Embankment Alternative 5

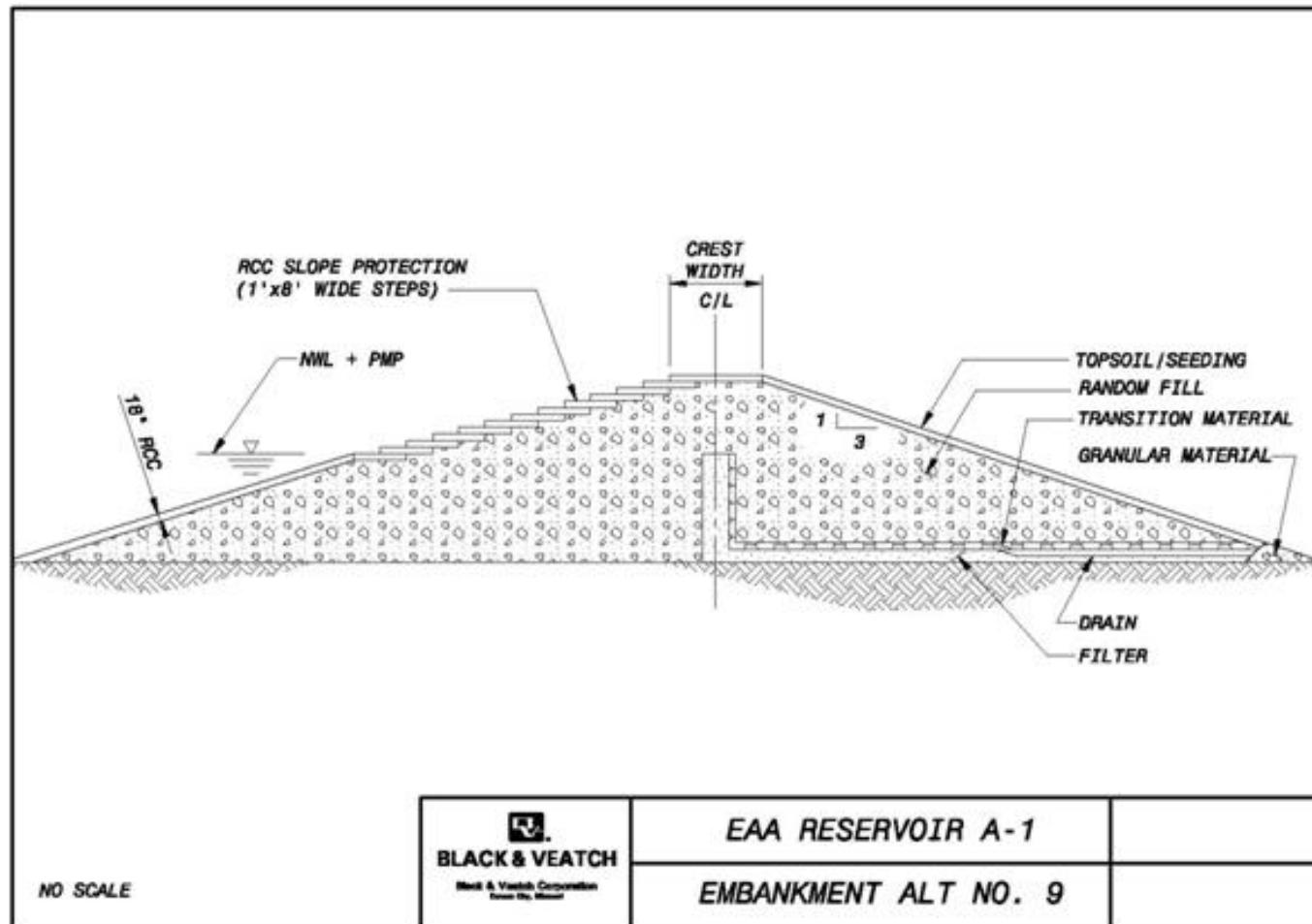
Figure 8 Embankment Alternative 8



## RCC Stability Analysis Embankment Alternative 5

Figure 9 Embankment Alternative 9

A5000L



## RCC Stability Analysis Embankment Alternative 5

Figure 10 Embankment Alternative 10

A5000L

